

December 20, 2022 Revised January 9, 2023 Project No. **19633.000.002**

Mr. Steve Arthur Pacific Gateway CA, LLC c/o Ridgeline Property Group 915 Highland Pointe Drive, Suite 250 Roseville, CA 95678

Subject: Pacific Gateway Tracy, California

DOUBLE RING INFILTRATION TESTING RESULTS

Dear Mr. Arthur:

As requested, we performed double-ring infiltration testing at the subject property in Tracy, California. The purpose of our services was to provide information pertinent to the design of the proposed project basins. A discussion of our field services and the results of our infiltration testing are provided below.

INFILTRATION FIELD TESTING

At each of the five proposed basin locations, we retained the services of a subcontractor to drill one boring to a depth of 20 feet. We reviewed subsurface conditions at each location and selected the most appropriate elevations for infiltration testing. We performed our double-ring tests at the approximate depth below the existing surface, as shown on Table 1 below. An ENGEO representative conducted two infiltration tests at various elevations within each of the five basin locations. The double-ring infiltration tests were performed in general conformance with ASTM D3385-18 and the Multi-Agency Post-Construction Stormwater Standards Manual. The approximate locations of our geotechnical borings and infiltration tests are provided in the attached figures.

The infiltration test maintains a constant head within the rings. Specialized Mariotte Tubes were used to maintain the water levels at the selected head elevation in both the inner ring and the annular space throughout the test. A constant head was achieved in DR-7 and DR-8 by manually adding a selected amount of water over a measured time interval due to the rapid infiltration rates of the targeted soil layers. Each infiltration test was run until the infiltration rate stabilized.

INFILTRATION TEST RESULTS

The infiltration rate for the double-ring infiltrometer was calculated using the following equation from ASTM D3385.

$$VIR = \Delta VIR / (AIR * \Delta t)$$

Where:

- VIR = inner ring incremental infiltration velocity, cm/hr
- Δ VIR = volume of liquid used during time interval to maintain constant head in the inner ring, cm³
- AIR = interior area of inner ring, cm^2

 $\Delta t =$ time interval, h

Pacific Gateway CA, LLC Pacific Gateway, Tracy, California DOUBLE RING INFILTRATION TESTING RESULTS

Based on the encountered soil types, the site soil would be anticipated to have infiltration rates varying from Type A to Type D soil, as presented in Table 3-1 of the Multi-Agency Post Construction Stormwater Standards Manual. Our double-ring infiltration test results are summarized in Table 1 below, along with a description of the soil type at each test location.

TABLE 1: Double-Ring Infiltrometer Test Results

TEST LOCATION	BASIN IDENTIFICATION	DEPTH (Below the existing ground surface, ft)	SOIL TYPE	INFILTRATION RATE (inches/hour)
DR-1	Basin 1	asin 1 7 Sandy silt		1.8
DR-2	Basin 1	asin 1 12 Silty sand		1.0
DR-3	Basin 2 5 Lean clay		0.3	
DR-4	DR-4 Basin 2 11½ Sandy lean clay		1.2	
DR-5	Basin 3	3	Lean clay	0.3
DR-6	Basin 3	91⁄2	Sandy lean clay	0.9
DR-7	Basin 4	9	Poorly graded gravel with silt and sand	15.0
DR-8	Basin 4	12	Poorly graded gravel with clay and sand	15.0
DR-9	Basin 5	4	Lean clay with sand	0.5
DR-10	Basin 5	9	Sandy silt	0.8

We strived to perform our professional services in accordance with generally accepted principles and practices currently employed in the area, there is no warranty, express or implied. If you have any questions regarding the contents of this letter, please do not hesitate to contact us.

Sincerely,

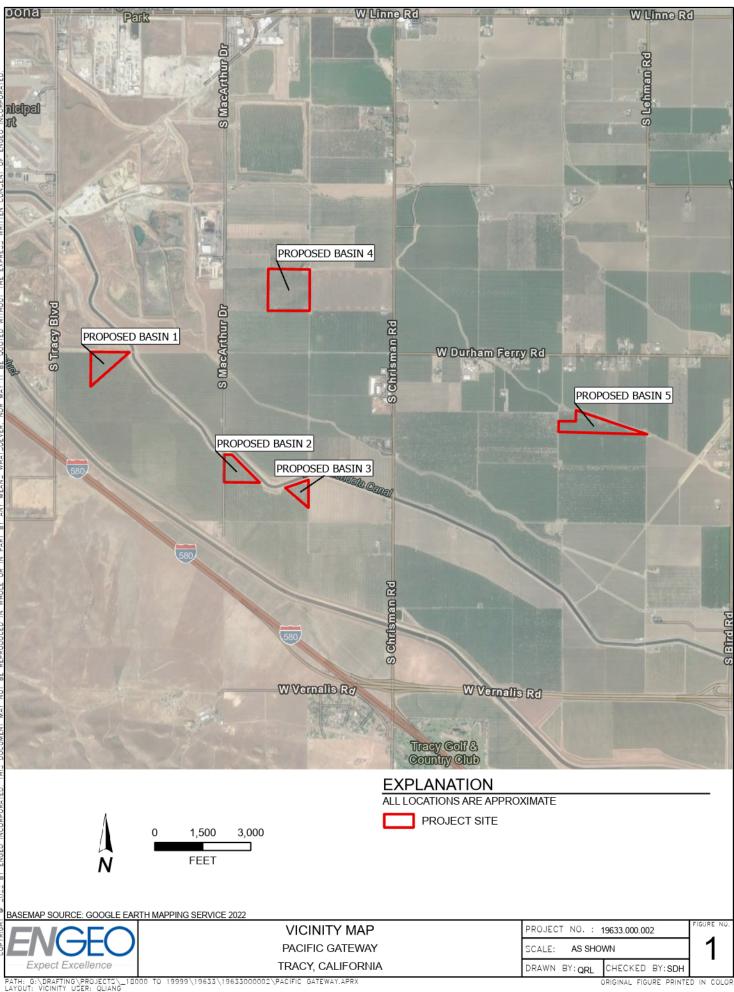
ENGEO Incorporated

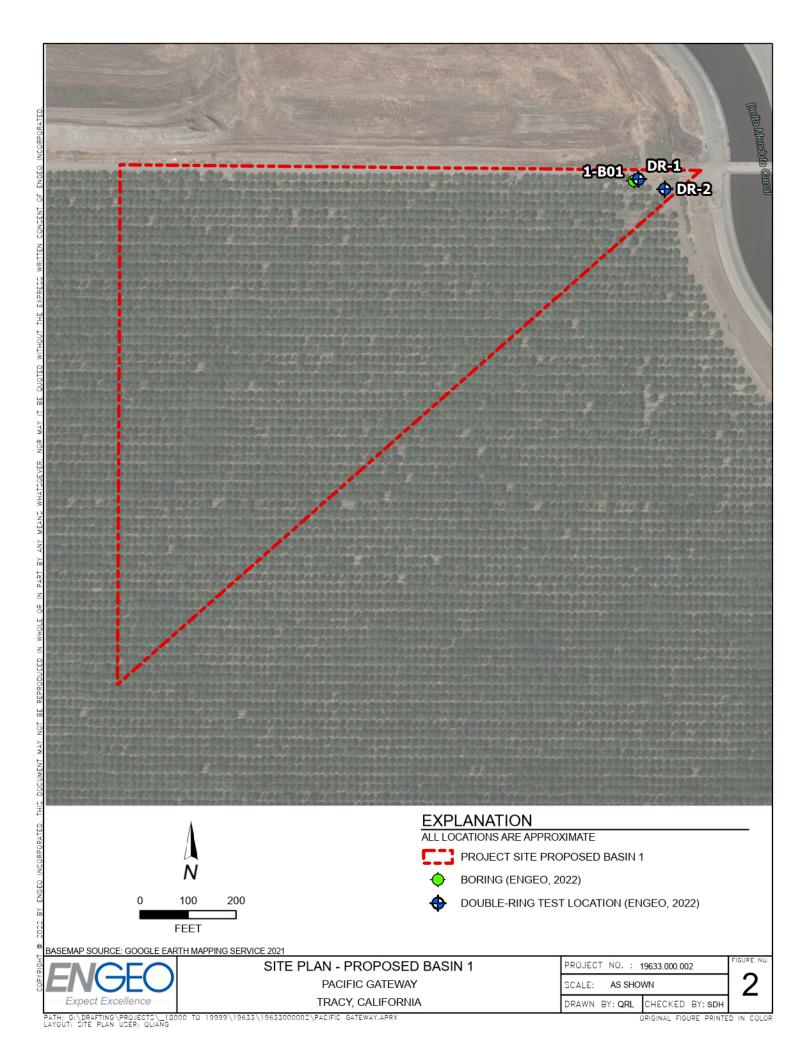
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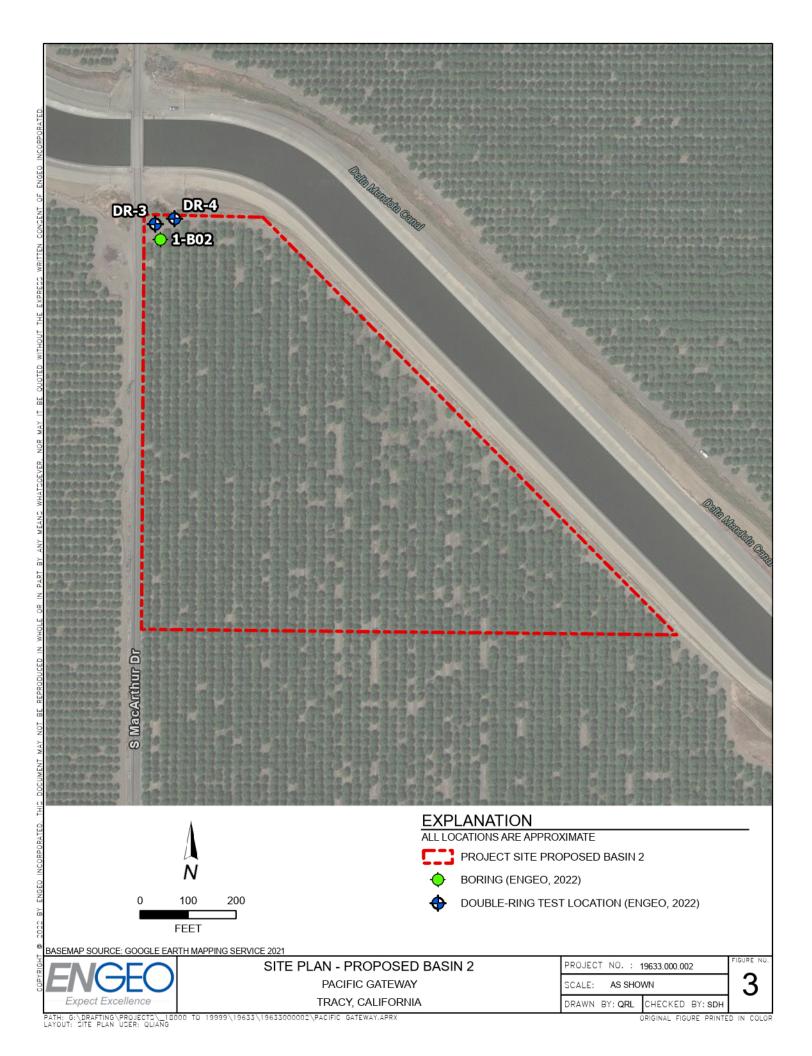
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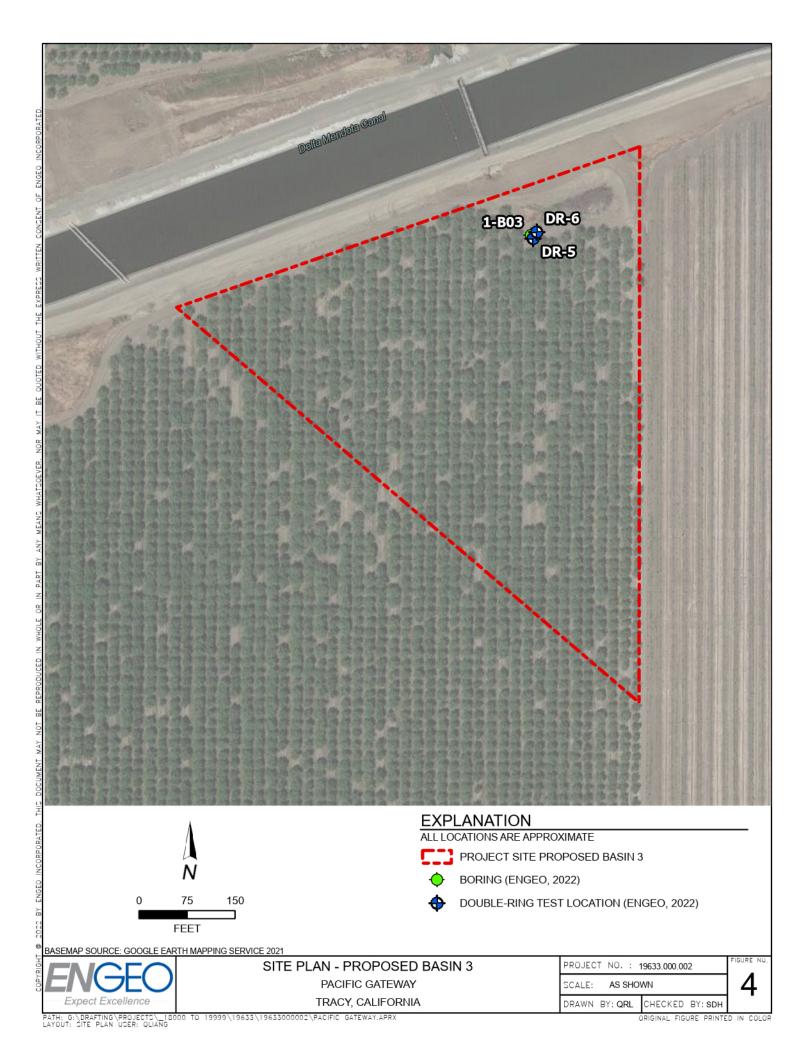
Attachments: Figure 1 – Vicinity Map Figure 2 – Site Plan - Proposed Basin 1 Figure 3 – Site Plan - Proposed Basin 2 Figure 4 – Site Plan - Proposed Basin 3 Figure 5 – Site Plan - Proposed Basin 4 Figure 6 – Site Plan - Proposed Basin 5 Boring Logs 1 through 5

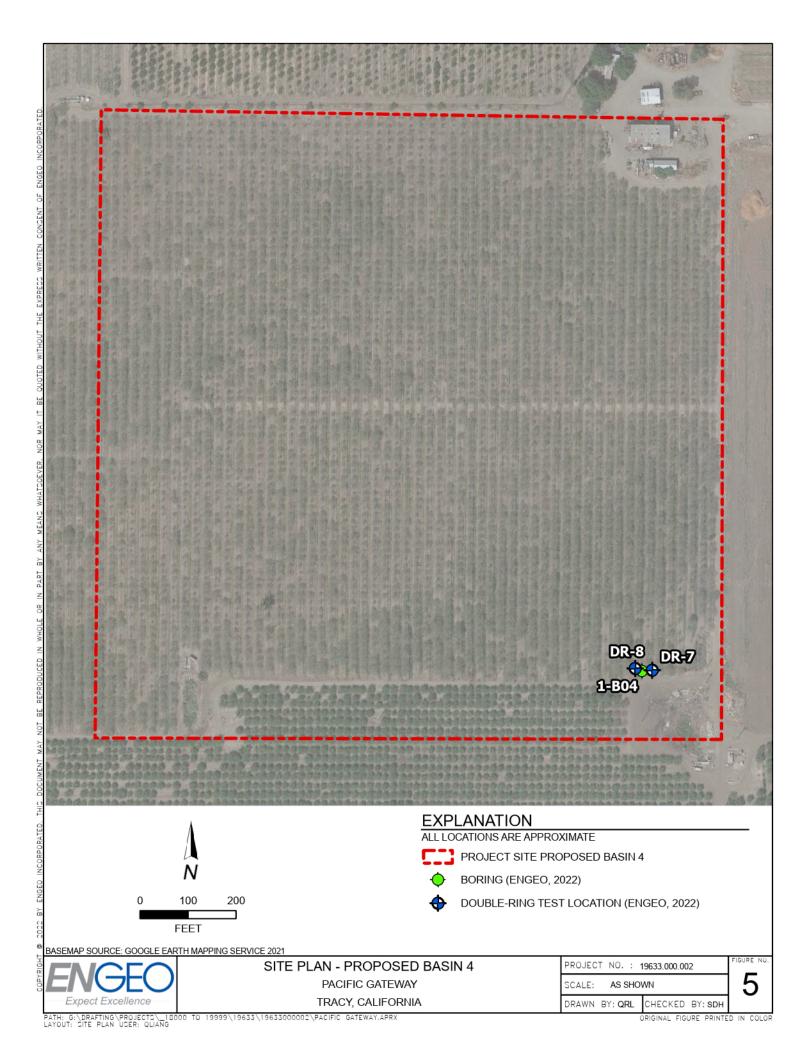
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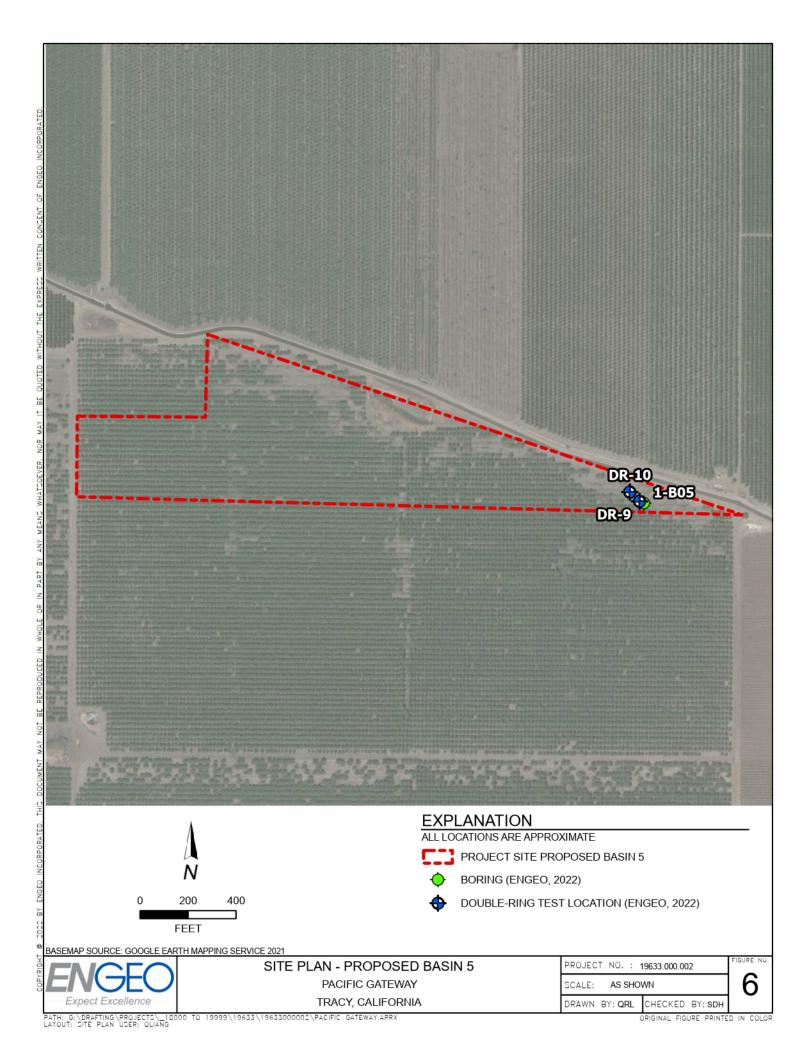












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	MAJO	R TYPES	KEY	TO BORIN	G LO	GS DESCRIPTIO	N	
Е ТНАN N #200	GRAVELS MORE THAN HALF COARSE FRACTION		AVELS WITH N 5% FINES	70	-	d gravels or gravel-sa ed gravels or gravel-s		s
COARSE-GRAINED SOILS MORE THAN HALF OF MAT'L LARGER THAN #200 SIEVE	IS LARGER THAN NO. 4 SIEVE SIZE	GRAVELS V 12	WITH OVER % FINES	GM - Silty	gravels	s, gravel-sand and sil	t mixtures	
GRAINED S = MAT'L LAI SIEV	SANDS MORE THAN HALF COARSE FRACTION IS SMALLER THAN		ANDS WITH N 5% FINES		-	d sands, or gravelly s ed sands or gravelly s		
COARSE- HALF OF	NO. 4 SIEVE SIZE		/ITH OVER % FINES	7.)		and-silt mixtures I, sand-clay mixtures		
OILS MORE NTL SMALLER SIEVE	SILTS AND CLAYS LIQ	UID LIMIT 50 %	OR LESS	CL - Inorga	anic cla	t with low to medium ay with low to mediun ay organic silts and cl	n plasticity	
FINE-GRAINED SOILS MORE THAN HALF OF MAT'L SMALLER THAN #200 SIEVE	SILTS AND CLAYS LIQUIE	D LIMIT GREATE	ER THAN 50 %	MH - Elast CH - Fat c	ic silt v lay with	vith high plasticity high plasticity ic organic silts and cl		
	HIGHLY OR	GANIC SOILS ed on the #200 siev	e, the words "with sand"	PT - Peat a	and oth	ner highly organic soi	ls	
For fin	e-grained soil with >30% retained on	the #200 sieve, the	e words "sandy" or "grav	elly" (whichever is predo	minant) are	e added to the group name.		
	<b>U.S. STANDARD</b> 200 40			RAIN SIZES	С	LEAR SQUARE SIEV 4 "	E OPENING	S 2"
SILT	S	SAND	0	4		VEL		2
ANE CLAY		MEDIUM	COARSE	FINE		COARSE	COBBLES	BOULDERS
	RELATI SANDS AND GRAVEL	VE DENSIT	Ύ LOWS/FOOT			CONSIST SILTS AND CLAYS	ENCY <u>STRENGTH*</u>	
	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	<u></u>	(S.P.T.) 0-4 4-10 10-30 30-50 OVER 50			VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	0-1/4 1/4-1/2 1/2-1 1-2 2-4 OVER 4	
				MOIS		CONDITION		
		SYMBOLS alifornia (3" O.E 2.5" O.D.) samp		DRY MOIST WET	Dam	Dusty, dry to touch p but no visible water ble freewater		
		plit spoon sam		LINE TYPE	S			
	Shelby Tube				Sc	blid - Layer Break		
		Moore Piston			Da	ashed - Gradational or a	oproximate laye	r break
	Continuous 0	Core		GROUNDWA	TER SY	MBOLS		
	Bag Samples	S		∑ ▼		ndwater level during drillin	g	
	🕅 Grab Sampl	es		Ţ	Stabi	lized groundwater level		
	NR No Recovery	/						
	S.P.T.) Number of blows of 140 lb	-						

* Unconfined compressive strength in tons/sq. ft., asterisk on log means determined by pocket penetrometer

<b>ENGEO</b>	
Expect Excellence	

	Expect Excellence LATITUDE						LONGITUDE: -121.426855										
G	Pa Tra	acifi acy	ical Exploration c Gateway , California 3.000.002	DATE DRILLED: HOLE DEPTH: HOLE DIAMETER: SURF ELEV (WGS84):	Approx. 21 4.5 in.	1∕2 ft.	DRILLING METHOD: Solid Flight Auger HAMMER TYPE: 140 lb. Rope and Cathe										
								Atter	berg L	imits	(e			<u> </u>	tsf)		
Depth in Feet	Elevation in Feet	Sample Type	DESC	Log Symbol	Water Level	Blow Count/Foot	Liquid Limit	Plastic Limit	Plasticity Index	Fines Content (% passing #200 sieve)	Moisture Content (% dry weight)	Dry Unit Weight (pcf)	Shear Strength (psf) *field approximation	Unconfined Strength (tsf) *field approximation	Strength Test Type		
-			SILTY SAND (SM), brown, fine-grained sand, 30-40%				28										
_	_		plasticity, 30-40% fine-grai	brown, stiff, moist, medium ned sand /n, loose, moist, fine-grained			12										
5 —	— 190 —						19										
-	_		SANDY SILT (ML), brown, non-plastic, 35-45% fine-g			<u></u>	10										
	— — 185 —		SILTY SAND (SM), brown, fine-grained sand, 30-40%	medium dense, moist, fines			16										
	— — — 180 —		(SP-SM), brown, medium of	WITH SILT AND GRAVEL Jense, moist, fine- to % fines, 30-40% fine to coarse			23										
20 —	  175		SILTY SAND WITH GRAV dense, moist, fine- to coars 20-30% fine to coarse grav	e-grained sand, 12-20% fines			17										
			Bottom of boring at approxi existing grade. Groundwate drilling.	mately 21 1/2 feet below er not encountered during													

<b>ENGEO</b>	
Expect Excellence	

LATITUDE: 37.658188

LONGITUDE: -121.41592

							:: 37.658188 LONGITUDE: -121.41592										
	Pa Tra	acif acy	ical Exploration ic Gateway /, California 33.000.002	DATE DRILLED: HOLE DEPTH: HOLE DIAMETER: SURF ELEV (WGS84):	Approx. 21 4.5 in.	1∕₂ ft.	DRILLING METHOD: Solid Flight Auger HAMMER TYPE: 140 lb. Rope and Cathead										
Depth in Feet	Elevation in Feet	Sample Type	DESC	RIPTION	Log Symbol	Water Level	Blow Count/Foot	Atter	Plastic Limit	Plasticity Index stim	Fines Content (% passing #200 sieve)	Moisture Content (% dry weight)	Dry Unit Weight (pcf)	Shear Strength (psf) *field approximation	Unconfined Strength (tsf) *field approximation	Strength Test Type	
-	- 185		LEAN CLAY (CL), dark bro medium plasticity, <15% fil carbonates	wn, very stiff to hard, moist, ne-grained sand, contains			40 22								>4.5*	PP	
5							73 26								>4.5*	PP	
10 -	180 		LEAN CLAY WITH SAND medium plasticity, 15-25%	fine-grained sand brown, hard, moist, low			68								>4.5*	PP	
	175 		plasticity, 35-45% fine-grai	 dense, moist, fine- to			30										
	170		SILTY SAND WITH GRAV moist, fine- to coarse-grain fine to coarse gravel	EL (SM), brown, very dense, ed sand, 15-25% fines, 20-309	~												
- 20 – 20 – 20 – 20 – 20 – 20 – 20 – 20			Detters of horize at approx	metely 24.4/2 feet heley			55										
			Bottom of boring at approx existing grade. Groundwate drilling.	mately 21 1/2 feet below er not encountered during													

<b>ENGEO</b>	
Expect Excellence	

			t Excellence	LATITUDE: 37.655847					LONGITUDE: -121.407512											
G	Pa Tra	acifi acy	ical Exploration c Gateway , California 3.000.002	HOLE DEPTH: HOLE DIAMETER:								LOGGED / REVIEWED BY: CM. Dunn / ZAC DRILLING CONTRACTOR: West Coast Exploration DRILLING METHOD: Solid Flight Auger HAMMER TYPE: 140 lb. Rope and Cathead								
Depth in Feet	Elevation in Feet	Sample Type	DESC	CRIPTION	Log Symbol	Water Level	Blow Count/Foot	Atter	Plastic Limit	Plasticity Index stimi	Fines Content (% passing #200 sieve)	Moisture Content (% dry weight)	Dry Unit Weight (pcf)	Shear Strength (psf) *field approximation	Unconfined Strength (tsf) *field approximation	Strenath Test Type				
<u> </u>	 190	Sa	LEAN CLAY (CL), dark gra medium to high plasticity, contains manganese nodu	Loo	M.	22	Lic	Pi	Pl	Fir (%	W)	Dr (p.	fi Sh	u∩ >4.5*	τ. PI					
5 —							13								4.25*	P				
-	185 		LEAN CLAY WITH SAND medium plasticity, 20-29%	(CL), brown, very stiff, moist, fine-grained sand			26 18								>4.5* 4.0*	P				
- 10 —			SANDY LEAN CLAY (CL), plasticity, 30-40% fine-grai	brown, hard, moist, medium ned sand			30								>4.5*	P				
-	— 180 —		grades to increasing sands	content, contains gravel																
15 —	— — — 175		LEAN CLAY WITH SAND medium plasticity, 15-25%	(CL), brown, stiff, moist, fine-grained sand			15													
- 20 -			grades to hard				33													
-			Bottom of boring at approx existing grade. Groundwate drilling.	mately 21 1/2 feet below er not encountered during																



		Exp	ect	Excellence	37.671124 LONGITUDE: -121.407554													
	Ģ	Pa Tra	icifi acy	ical Exploration c Gateway , California 3.000.002	HOLE DEPTH: A HOLE DIAMETER: 4	ATE DRILLED: 12/2/2022 HOLE DEPTH: Approx. 21½ ft. .E DIAMETER: 4.5 in. .EV (WGS84): Approx. 148 ft.					DRILLING METHOD: Solid Flight Aug							
	Depth in Feet	Elevation in Feet	Sample Type	DESC	Log Symbol	Water Level	Blow Count/Foot	Liquid Limit	Plastic Limit	Plasticity Index stim	Fines Content (% passing #200 sieve)	Moisture Content (% dry weight)	Dry Unit Weight (pcf)	Shear Strength (psf) *field approximation	Unconfined Strength (tsf) *field approximation	Strength Test Type		
-			Sar	LEAN CLAY (CL), dark gra medium to high plasticity, <	Pog	Wa	29	Liqu	Pla	Pla	Fine (% p	Mo (%)	(pc:					
	-	145 		LEAN CLAY WITH SAND moist, medium plasticity, 1	(CL), brown, medium stiff, 5-25% fine-grained sand			6							1750*	3.5	PP+TV	
	5 —	-		grades to very stiff SANDY LEAN CLAY (CL), medium plasticity, 35-45%				19								2.75*	PP	
	-	140			EL WITH SILT AND SAND			16								>4.5*	PP	
	10 — - -	-		coarse gravel, 5-12% fines coarse-grained sand	, 35-45% fine- to		01 01	53										
C.GDT 12/20/22	- 15 —	— 135 — —		(GP-GC), yellowish brown,	EL WITH CLAY AND SAND dense, moist, fine to coarse % fine- to coarse-grained sand			30										
1_1-B05.GPJ ENGEO IN	-	130		CLAYEY SAND (SC), brow fine-grained sand, 40-49%														
W/ ELEV 1-B0	20 —	-						19										
LOG - GEOTECHNICAL_SU+QU W/ ELEV 1-B01_1-B05.GPJ ENGEO INC.GDT 12/20/				Bottom of boring at approxi existing grade. Groundwate drilling.														
- 901																		

ENGE	0
Expect Exceller	nce

		— Exp	eci	t Excellence	37.660539 LONGITUDE: -121.371812													
	G	Pa Tra	acifi acy	ical Exploration c Gateway , California 3.000.002	DATE DRILLED: 12 HOLE DEPTH: Ap HOLE DIAMETER: 4. SURF ELEV (WGS84): Ap	prox. 2 5 in.	1½		DRILLING METHOD: Solid Flight Auger HAMMER TYPE: 140 lb. Rope and Cathead									
	Depth in Feet	Elevation in Feet	Sample Type	DESC	Log Symbol		Water Level Blow Count/Foot		Plastic Limit	Plasticity Index	Fines Content (% passing #200 sieve)	Moisture Content (% dry weight)	Dry Unit Weight (pcf)	Shear Strength (psf) *field approximation	Unconfined Strength (tsf) *field approximation	Strength Test Type		
С.GDT 12/20/22	<ul> <li>■</li> <li>-</li> <li>-</li></ul>	<u> </u>	Sa	medium to high plasticity, contains carbonates grades to brown, 20-29% f LEAN CLAY (CL), brown, plasticity, <15% fine-graine LEAN CLAY WITH SAND medium plasticity, 15-25% SANDY SILT (ML), brown, non-plastic, 30-40% fine-g grades to medium stiff	ine-grained sand hard, moist, medium to high ed sand (CL), brown, very stiff, moist, fine-grained sand stiff, moist, low plasticity to rained sand H), brown, stiff, moist, medium			5	8 8 3 6			Ein (%)	MG (%)		Sh *fic	>4.5* >4.5* >4.5* 2.75*	- 역 · · · · · · · · · · · · · · · · · ·	
LOG - GEOTECHNICAL_SU+QU W/ ELEV 1-B01_1-B05.GPJ ENGEO INC.GDT 12/20	- - 20 — -	110 110		Bottom of boring at approx existing grade. Groundwate drilling.	imately 21 1/2 feet below er not encountered during			1	1									